

November 30, 2023

Independence Public Works
Attn: Mr. Rich Kemple
Deputy Director Municipal Services
111 E. Maple Ave.
Independence, MO 64050

Re: **Structural Inspection Cracked and Dislodged Brick Tile Veneer at Front Entrance**

Dear Mr. Kemple,

In late August this year, at your request, we inspected cracked and dislodged brick tile veneer near the front entrance of the Independence City Hall building. This inspection follows our previous inspection of several cracked windowpanes located throughout the building's perimeter façade. This previous inspection took place in November 2020 and is attached for reference (less Exhibit B – Structural Analysis). The current matter with the cracking of the brick tile veneer is attributed to a couple of issues, as described below.

One matter we identified is the minimal or lack of sufficient control joints near the corner of the where the cracked brick veneer has taken place. Additionally, 'Creep' remains a part of the current issue. With Creep, *the time-dependent deformation in concrete due to sustained loads*, a slight translation at the beam – to – column structural interface can place undue stress on non-structural components (brick veneer). This undue stress on the non-structural components will occur if there is a lack of control joints – as we have in this case.

There is no indication that the structural framing of the building is compromised. The ongoing issue with the concrete 'creep' will need to be addressed with respect to the windows. Please note the following recommendation as presented in the November 2020 report:

Our recommendation for permanent repairs is the replacement of the window system where the distress/cracks in glazing is present. The replacement should comprise the entire bank of windows where a new head receptor would allow for the current parabolic profile of the deflected wall system above.

We've estimated the costs to replace all the windows in the building to be \$1,200,000.00.

We recommend the repairs of the cracked brick tile veneer include placement of wider control joints.

Please do not hesitate to call or email us if you have any questions or require further information.

Very truly yours,



Gerald L. Charlton

Enc.

November 3, 2020

Independence Public Works
Attn: Mr. Michael Schott
Facilities/Fleet Manager
111 E. Maple Ave.
Independence, MO 64050

Re: **Structural Inspection Report and Repair Recommendation of Cracked Glazing**

Dear Mr. Schott,

We have completed our Structural Analysis of the City Hall Building, located at 111E. Maple Avenue. The structural analysis was performed as part of the City of Independence Purchase Order # 20001086. The work was described in Exhibit 'A' of our proposal for Professional Services, dated July 10, 2010 and further outlined under Phase 100; specifically:

Task 101: Structural Inspection

RIC will perform a structural inspection of the City Hall office building, limited to the cause of the distress/cracking of the glazing describe in the introduction of this letter agreement. As part of the inspection, RIC will take physical measurements of the structural framing system at the buildings perimeter. The measurements will allow us to examine whether concrete 'creep' is causing a serviceability issue. Our findings will be presented in a report.

In order to perform and model the above described analysis, a thorough survey was performed for creation of a structural framing plan for each elevated floor. The framing plans are presented in Exhibit 'A' and are comprised of the following:

1. 2nd Floor Framing Plan
2. 3rd Floor Framing Plan
3. Roof Framing Plan

The structure can be described as a cast-in-place concrete column and concrete pan joist framing system. A waffle slab framing system was incorporated in areas of each floor for an aesthetic appearance. These areas are located as follows and are shown in Exhibit 'A' Framing Plans:

- 2nd Floor Waffle Slab appears in ceiling of the Court Room area & north ground floor lobby
- 3rd Floor Waffle Slab appears in ceiling of 2nd floor main lobby
- Roof Waffle Slab appears in ceiling of 3rd floor lobby

The perimeter concrete beams supporting the integral pan joist floor system were examined for the purpose of locating cracked sections as part of our structural analysis. No cracked sections were discovered.

Our analysis focused on the perimeter framing system in order to determine serviceable deflections. Structural framing systems are designed to avoid excessive deformation/deflection which may lead to serviceable issues, such as – unsightly sagging of floors and roofs, cracking and other forms of distress to nonstructural components. Buildings with brick veneer exterior walls are typically limited to deflections having a ratio of $l/600$, with l = span length or column height.

The analysis results are part of Exhibit ‘B’. In addition to the Elevated Floor Framing Plans presented in Exhibit ‘A’, we included Building Elevations to better present the Serviceability Deflections that were derived from our analysis.

Additionally, we have identified creep as being a probable issue that has led to the distress/cracking in the glazing. During our initial inspection, we noticed that some window mullions presented significant bowing. This is typical of a compressive load. The original window installation was completed in a static environment – that being no external forces other than nominal wind loads. For the mullions to bow or deflect sideways dynamic forces are taking place. As mentioned earlier, we identified creep as a probable cause. Creep is the time-dependent deformation in concrete due to sustained loads. Long term creep is typically 2x’s the serviceability deflection of dead load.

The worst case serviceability deflection was located in the 2nd Floor perimeter beam supporting the east face brick veneer wall system – between Grid Line (B-C). The cracked window in the east face is located below this beam. The deflection measured in this beam is approximately 0.75 inches. The serviceable deflection in this beam is 0.5 inches. The total deflection supports our overview that long term creep is presenting the dynamic forces that is leading to the distress in the glazing. A total deflection of 0.75 inches indicates that the deflection due to creep is approximately 0.25 inches. The dynamic force present is a result of the above beam/wall system having a deflection greater than the allowable space in the window frames head receptor. We want to point out that the total serviceable + long-term creep deflection of 0.75 is within the $l/600$ criteria.

Task 102: Repair Recommendation

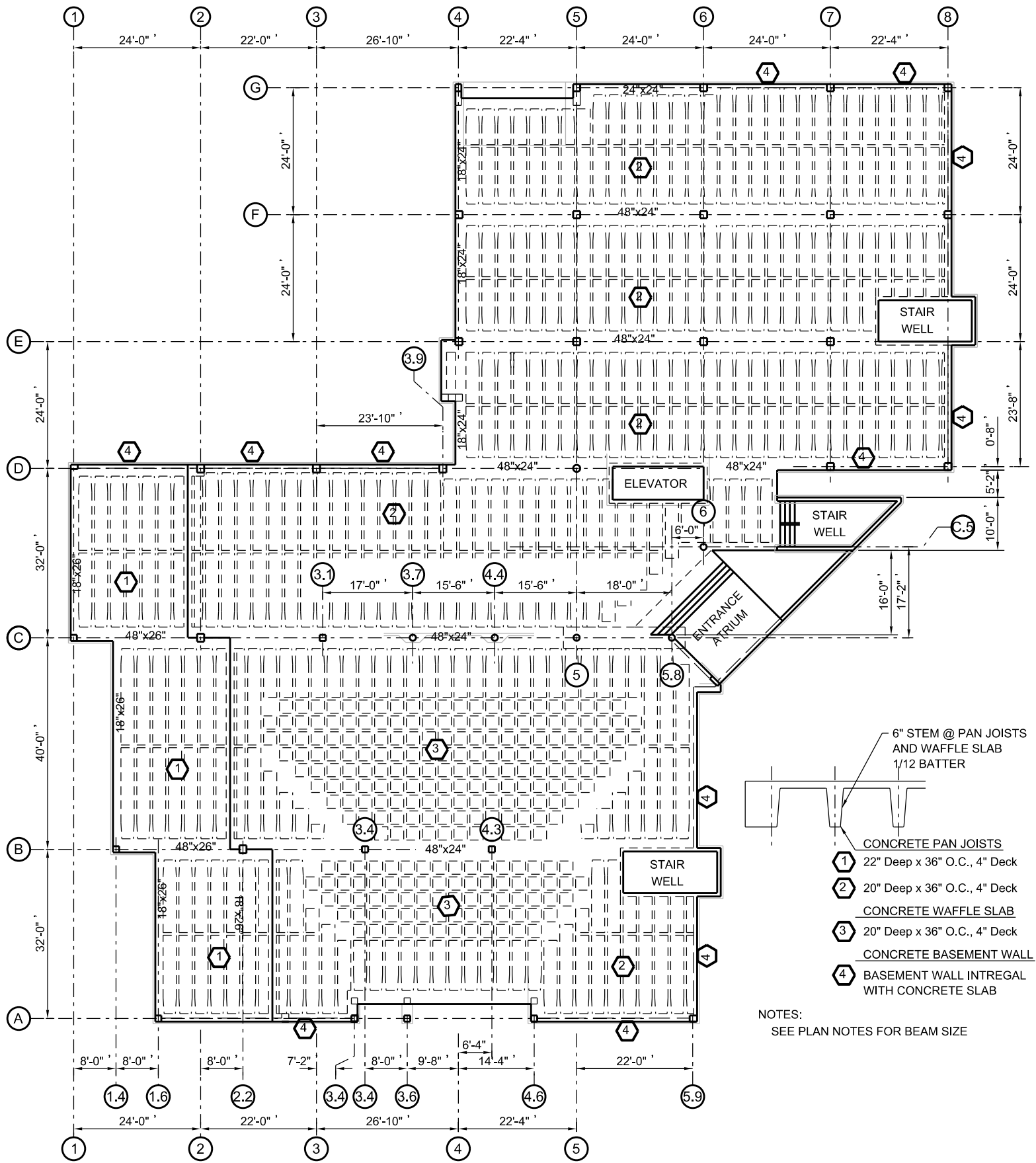
RIC will discuss our assessment with a glazing contractor/supplier in order to recommend necessary repairs/replacement. These recommendations will include preventative measures to minimize the possible distress in other perimeter glazing elements.

Other than the window cracking and bowed mullions, there does not appear to be any further distress in non-structural building components.

Temporarily, we recommend the City avoid storing heavy loads, such as filing cabinets or stacks of boxed hard copy documents near the perimeter of the building. This recommendation is for future office arrangements. Current office arrangements are fine.

Our recommendation for permanent repairs is the replacement of the window system where the distress/cracks in glazing is present. The replacement should comprise the entire bank of windows where a new head receptor would allow for the current parabolic profile of the deflected wall system above.

Appendix 'A'

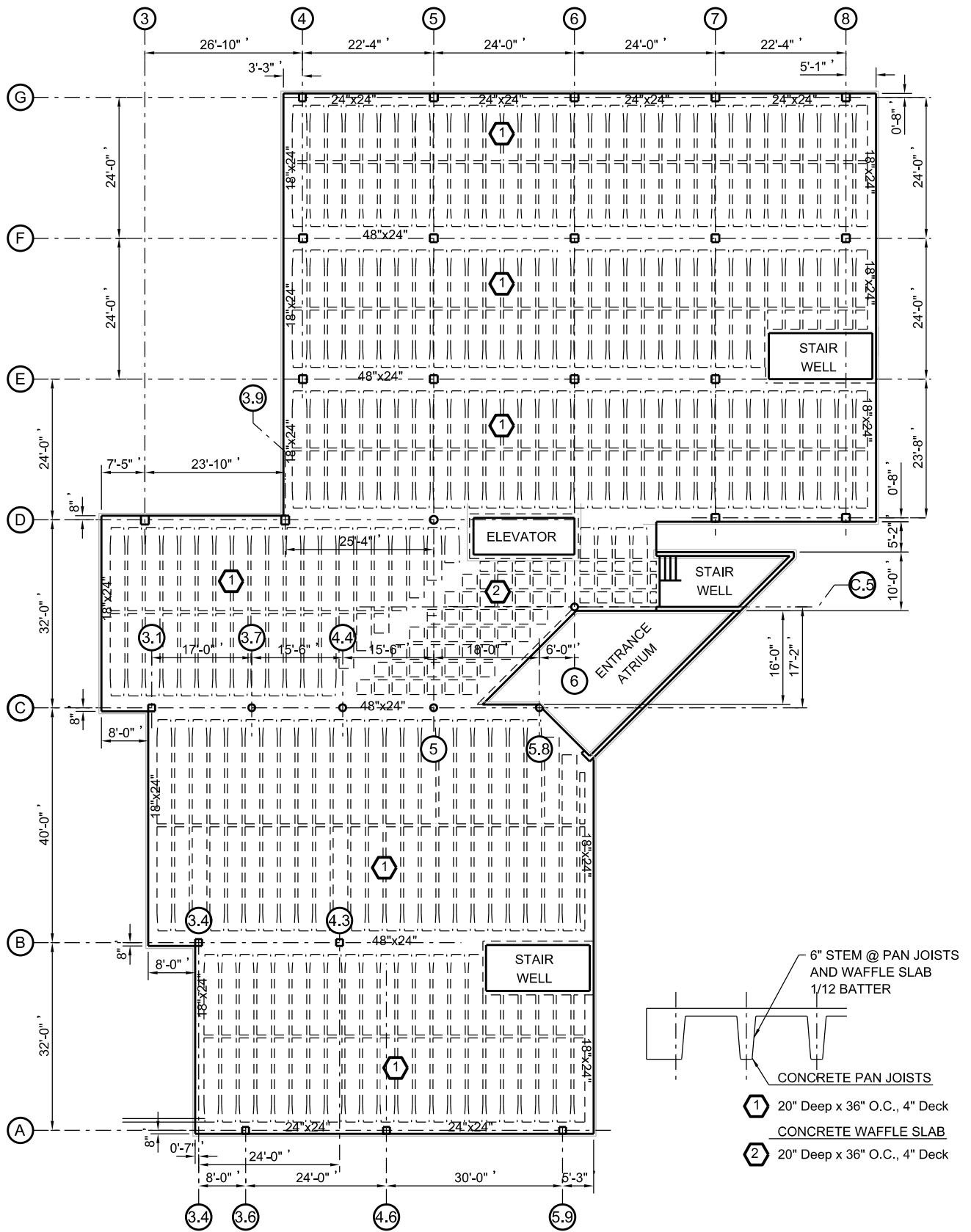


- 6" STEM @ PAN JOISTS AND WAFFLE SLAB 1/12 BATTER
- CONCRETE PAN JOISTS
- 1 22" Deep x 36" O.C., 4" Deck
- 2 20" Deep x 36" O.C., 4" Deck
- CONCRETE WAFFLE SLAB
- 3 20" Deep x 36" O.C., 4" Deck
- CONCRETE BASEMENT WALL
- 4 BASEMENT WALL INTEGRAL WITH CONCRETE SLAB

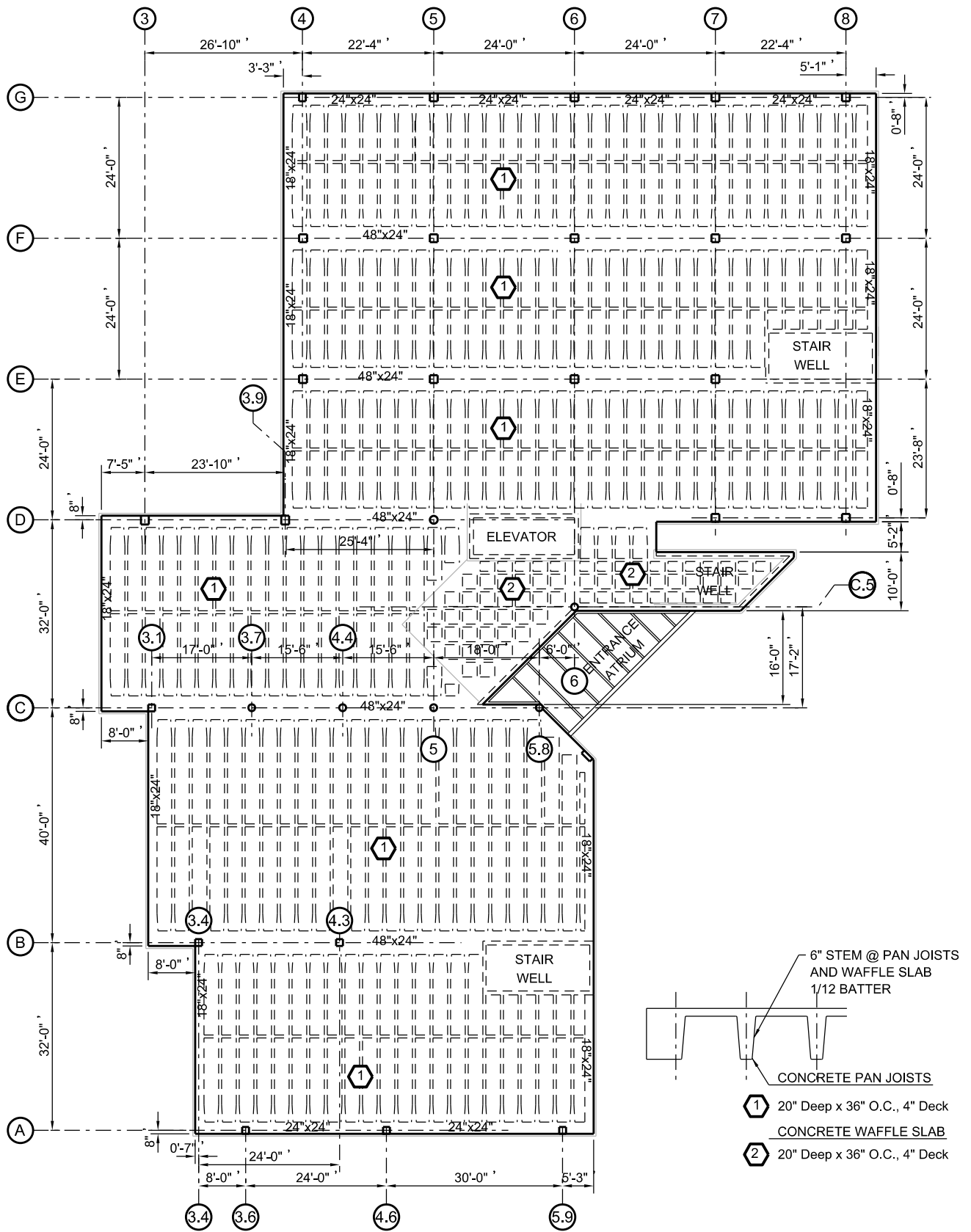
NOTES:
SEE PLAN NOTES FOR BEAM SIZE

INDEPENDENCE CITY HALL

2nd FLOOR FRAMING PLAN



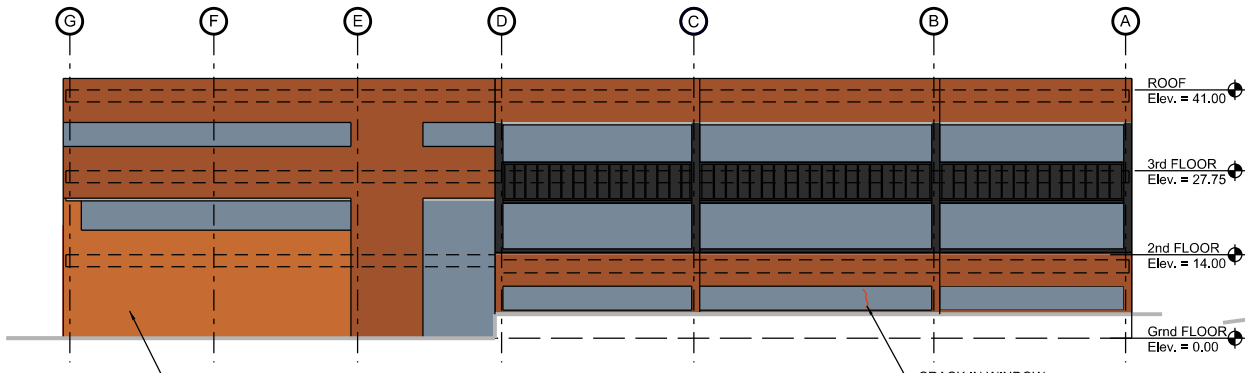
INDEPENDENCE CITY HALL
3rd FLOOR FRAMING PLAN



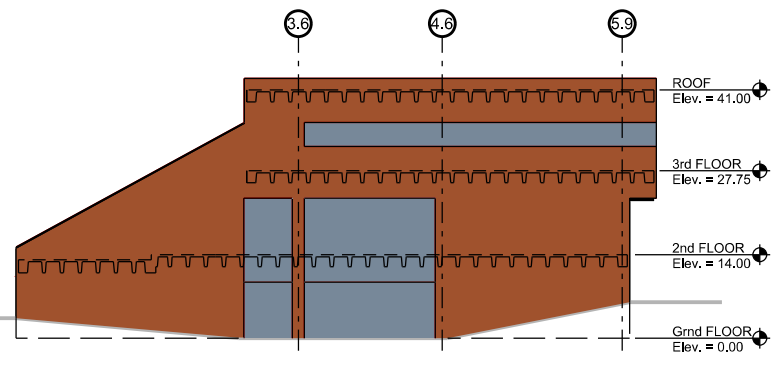
NOTES:
SEE PLAN NOTES FOR BEAM SIZE

INDEPENDENCE CITY HALL

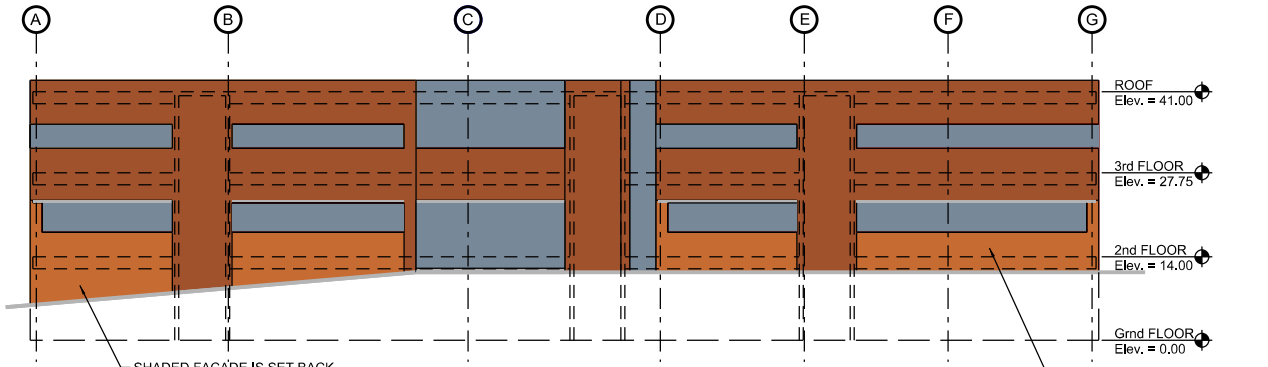
ROOF FRAMING PLAN



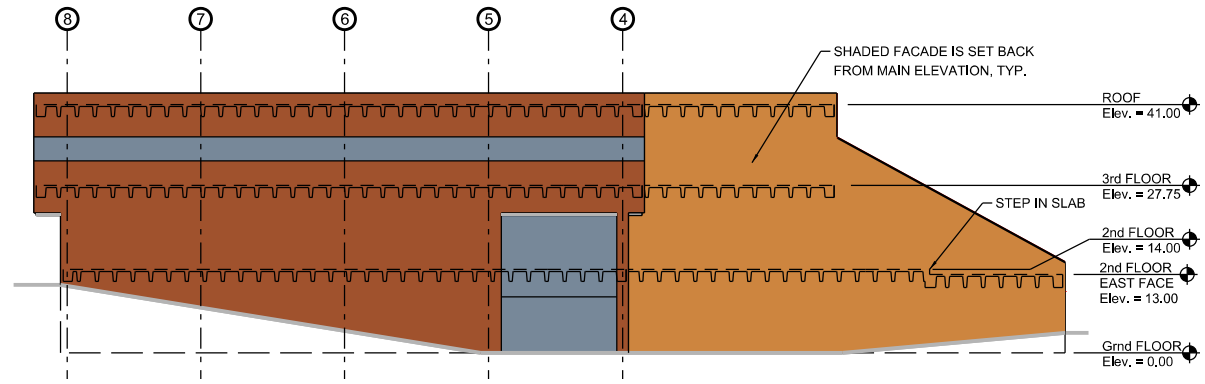
EAST ELEVATION



NORTH ELEVATION



WEST ELEVATION



SOUTH ELEVATION

Appendix 'B'

Roof Framing - East & West Face

Assume: $f'_c = 4,000 \text{ psi}$; $\gamma = 150 \text{ pcf}$

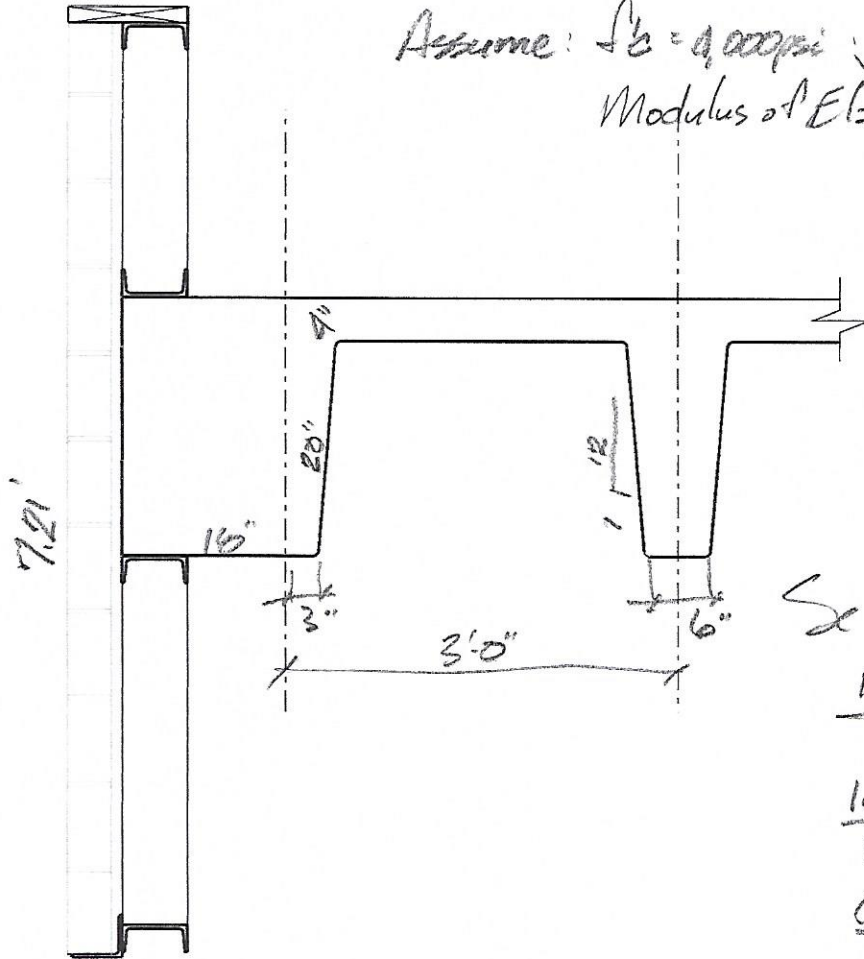
Modulus of Elasticity: $E_c = 33 \times \gamma^{1.5} \sqrt{f'_c}$

$$E_c = 33 \times 150^{1.5} \sqrt{4,000}$$

$$E_c = 3,834,253 \text{ psi}$$

$$I = \frac{bd^3}{12}$$

$$I = \frac{18 \times 24^3}{12} = 20,736 \text{ in}^4$$



Self Wt.

$$\frac{18 \times 24}{144} \times 150 = 450 \text{ pcf}$$

$$\frac{1.67 \times 20}{2 \times 144} \times 150 = 17 \text{ pcf}$$

$$\frac{(18-3) \times 4}{144} \times 150 = 18 \text{ pcf}$$

$$485 \text{ pcf}$$

Wt. Brick Facade: $7.21 \times 40 =$

$$288$$

Back-Up Framing: $(7.21 - 2) \times 10 =$

$$52$$

Ceiling / MEP: $3 \times 6 \text{ pcf} =$

$$18$$

$$943 \text{ pcf}$$

Roof Framing - North & South Face

Assume: $f'_c = 4,000 \text{ psi}$; $\gamma = 150 \text{ pcf}$

Modulus of Elasticity: $E_c = 23 \times \gamma^{1.5} \sqrt{f'_c}$

$$E_c = 23 \times 150^{1.5} \sqrt{4000}$$

$$E_c = 3,824,253 \text{ psi}$$

$$I = \frac{24 \times 24^3}{12} = 27,648 \text{ in}^4$$

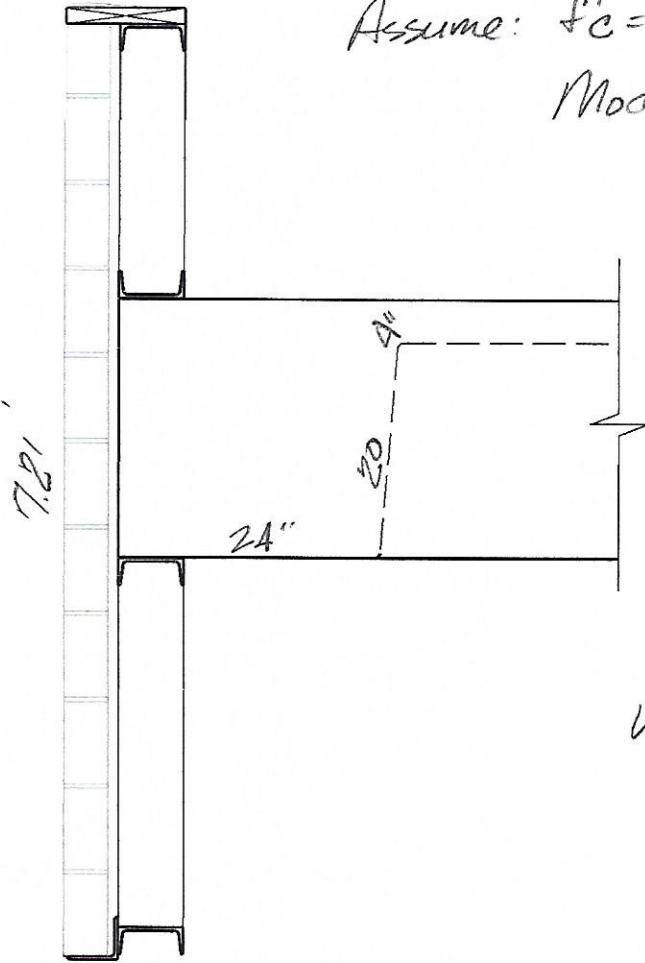
Self wt.:

$$\frac{24 \times 24}{144} \times 150 = 600 \text{ pcf}$$

$$\text{Wt. Brick Facade: } 7.21 \times 40 = 288 \text{ pcf}$$

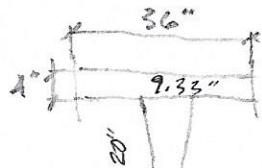
$$\text{Back-Up Frame: } (7.21 - 2) \times 10 = 52 \text{ pcf}$$

$$\text{Ceiling/MEP } 2 \times 6 = \frac{12}{952 \text{ pcf}}$$



Tributary Loading:

South Face Grid (F-G)



$$\frac{\left(\frac{9.33 + 6}{2}\right) \times 20}{144} \times 150 = 110.0 \text{ pcf}$$

$$\frac{4 \times 36}{144} \times 150 = 150.0 \text{ pcf}$$

$$\text{Cross Beam } 4 \times 12 \times 1.5 \times 150 = 300 \text{ lb}$$

$$\text{Ceiling/MEP } 3 \times 4 = 18 \text{ pcf}$$

$$w = \left[\frac{328 \times 20.67}{2} + \frac{300}{2} \right] \div 3 = 1,180 \text{ pcf}$$

$$\frac{952}{2,132 \text{ pcf or}}$$

North Face Grid (A-B)

$$w = \left[\frac{328 \times 20.58}{2} + \frac{300}{2} \right] \div 3$$

$$w = 1,613 \text{ pcf}$$

$$+ 952$$

$$\hline 2,565 \text{ pcf}$$

$$\text{or } 213.75 \text{ lb/in}$$

20.6667'
20.5833'

328

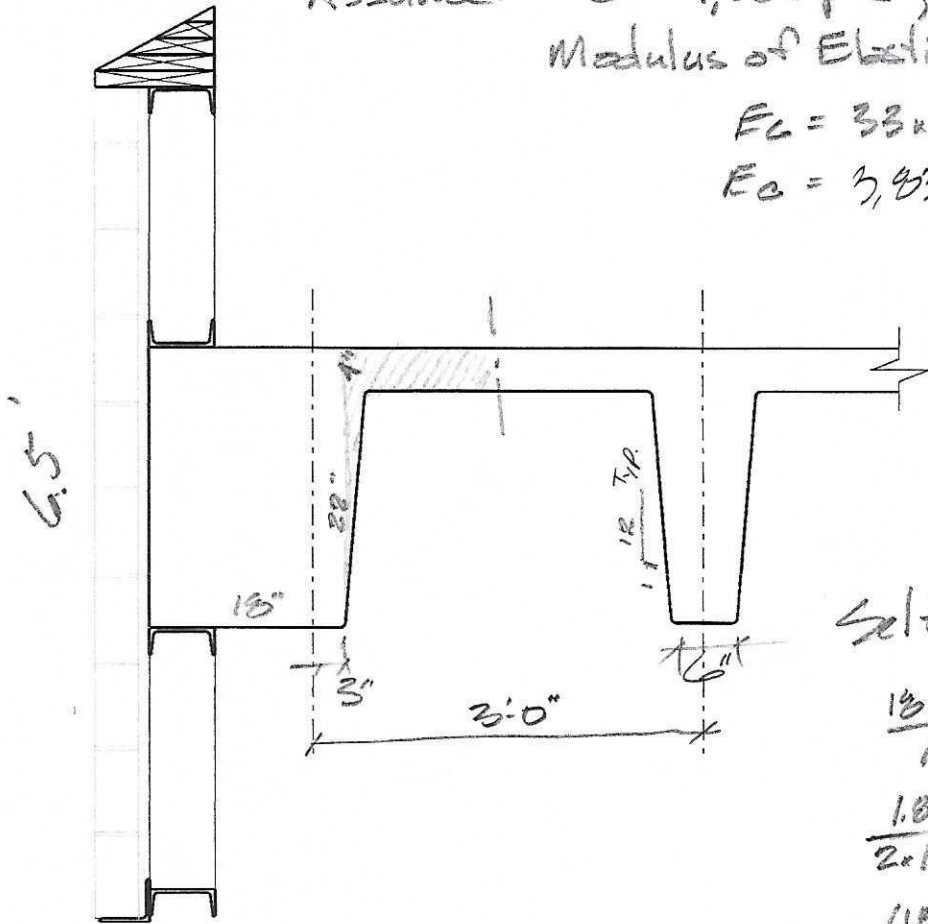
2nd Floor - East Face

Assume: $f'_c = 4,000 \text{ psi}$; $\gamma = 150 \text{ pcf}$

Modulus of Elasticity: $E_c = 33 \times 10^4 \sqrt{f'_c}$

$$E_c = 33 \times 10^4 \sqrt{4,000}$$

$$E_c = 3,934,253 \text{ psi}$$



$$I = \frac{bd^3}{12}$$

$$I = \frac{18 \times 26^3}{12}$$

$$I = 26,364 \text{ in}^4$$

Self wt.:

$$\frac{18 \times 26 \times 150}{144} = 487.5 \text{ pcf}$$

$$\frac{1.83 \times 22 \times 150}{2 \times 144} = 21.0 \text{ pcf}$$

$$\frac{(18-3) \times 9 \times 150}{144} = \frac{62.5 \text{ pcf}}{571 \text{ pcf}}$$

$$\text{Wt. Brick Facade: } 6.5 \times 40 \text{ pcf} = 260 \text{ pcf}$$

$$\text{Back up Framing: } (6.5 - 2.17) 10 = 43 \text{ pcf}$$

$$\text{Ceiling MEP: } 3 \times 6 \text{ pcf} = 18 \text{ pcf}$$

$$\underline{892 \text{ pcf}}$$

2nd Floor Framing – East Face: 1 Span (40ft); Supports (Pinned, Pinned)

$F'c = 4,000 \text{ psi}$

$E_c = 3,834,253 \text{ psi}$

$\delta = 150 \text{ lb/ft}^3 \text{ or } 0.0868 \text{ lb/in}^3$

$I = 26,364 \text{ in}^3$

$w = 892 \text{ lb/ft or } 74.33 \text{ lb/in}$

Force = [lb] or [lb*in]

Displacement = [in] or [rad]

Location = [in]

SUPPORT JOINT REACTIONS (in direction of rotated joint axes)

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JOINT	X-REACTION	Y-REACTION	Z-MOMENT
1	0.00000	17839.20000	0.00000
2	0.00000	17839.20000	0.00000

MEMBER INTERNAL FORCES (at even intervals along span length)

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SPAN 1	AXIAL	SHEAR	MOMENT
0	0.00000	17839.20000	0.00000
1	0.00000	14271.36000	770653.44000
2	0.00000	10703.52000	1370050.56000
3	0.00000	7135.68000	1798191.36000
4	0.00000	3567.84000	2055075.84000
5	0.00000	0.00000	2140704.00000

6	0.00000	-3567.84000	2055075.84000
7	0.00000	-7135.68000	1798191.36000
8	0.00000	-10703.52000	1370050.56000
9	0.00000	-14271.36000	770653.44000
10	0.00000	-17839.20000	0.00000

SUPPORT JOINT DISPLACEMENTS (in direction of rotated joint axes)

=====

JOINT	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
1	0.000000000	0.000000000	-0.003388321
2	0.000000000	0.000000000	0.003388321

MEMBER DISPLACEMENTS (at even intervals along span length)

=====

SPAN 1	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
0	0.000000000	0.000000000	-0.003388321
1	0.000000000	-0.159549254	-0.003198575
2	0.000000000	-0.301858732	-0.002683550
3	0.000000000	-0.413266723	-0.001924566
4	0.000000000	-0.484014863	-0.001002943
5	0.000000000	-0.508248135	0.000000000
6	0.000000000	-0.484014863	0.001002943
7	0.000000000	-0.413266723	0.001924566
8	0.000000000	-0.301858732	0.002683550
9	0.000000000	-0.159549254	0.003198575
10	0.000000000	0.000000000	0.003388321

MAXIMUM / MINIMUM SPAN SHEARS

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SPAN	MAX SHEAR	LOCATION	MIN SHEAR	LOCATION
1	17839.20000	0.0000	-17839.20000	480.0000

MAXIMUM / MINIMUM SPAN MOMENTS

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SPAN	MAX MOMENT	LOCATION	MIN MOMENT	LOCATION
1	2140704.00000	240.0000	0.00000	0.0000

MAXIMUM / MINIMUM VERTICAL SPAN DEFLECTIONS

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SPAN	MAX DEFLECTION	LOCATION	MIN DEFLECTION	LOCATION
1	0.000000000	0.0000	-0.508248135	240.0000 000

2nd Floor Framing

Perimeter Beam – East Face between (A-B) & (C-D)

Force = [lb] or [lb*in]

Displacement = [in] or [rad]

Location = [in]

SUPPORT JOINT REACTIONS (in direction of rotated joint axes)

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JOINT	X-REACTION	Y-REACTION	Z-MOMENT
1	0.00000	14592.00000	0.00000
2	0.00000	14592.00000	0.00000

MEMBER INTERNAL FORCES (at even intervals along span length)

=====

SPAN 1	AXIAL	SHEAR	MOMENT
0	0.00000	14592.00000	0.00000
1	0.00000	11673.60000	504299.52000
2	0.00000	8755.20000	896532.48000
3	0.00000	5836.80000	1176698.88000
4	0.00000	2918.40000	1344798.72000
5	0.00000	0.00000	1400832.00000
6	0.00000	-2918.40000	1344798.72000
7	0.00000	-5836.80000	1176698.88000
8	0.00000	-8755.20000	896532.48000
9	0.00000	-11673.60000	504299.52000

10 0.00000 -14592.00000 0.00000

SUPPORT JOINT DISPLACEMENTS (in direction of rotated joint axes)

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JOINT	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
1	0.000000000	0.000000000	-0.002255227
2	0.000000000	0.000000000	0.002255227

MEMBER DISPLACEMENTS (at even intervals along span length)

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SPAN 1	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
0	0.000000000	0.000000000	-0.002255227
1	0.000000000	-0.084955305	-0.002128934
2	0.000000000	-0.160730934	-0.001786140
3	0.000000000	-0.220052427	-0.001280969
4	0.000000000	-0.257723740	-0.000667547
5	0.000000000	-0.270627247	0.000000000
6	0.000000000	-0.257723740	0.000667547
7	0.000000000	-0.220052427	0.001280969
8	0.000000000	-0.160730934	0.001786140
9	0.000000000	-0.084955305	0.002128934
10	0.000000000	0.000000000	0.002255227

MAXIMUM / MINIMUM SPAN SHEARS

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SPAN	MAX SHEAR	LOCATION	MIN SHEAR	LOCATION
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1 14592.00000 0.0000 -14592.00000 384.0000

MAXIMUM / MINIMUM SPAN MOMENTS

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SPAN	MAX MOMENT	LOCATION	MIN MOMENT	LOCATION
1	1400832.00000	192.0000	0.00000	384.0000

MAXIMUM / MINIMUM VERTICAL SPAN DEFLECTIONS

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SPAN	MAX DEFLECTION	LOCATION	MIN DEFLECTION	LOCATION
1	0.000000000	0.0000	-0.270627247	192.0000

Roof Framing – East Face: 3 Spans @ 24ft

$F'c = 4,000 \text{ psi}$

$E_c = 3,834,253 \text{ psi}$

$\delta = 150 \text{ lb/ft}^3 \text{ or } 0.0868 \text{ lb/in}^3$

$I = 20,736 \text{ in}^3$

Force = [lb] or [lb*in]

Displacement = [in] or [rad]

Location = [in]

SUPPORT JOINT REACTIONS (in direction of rotated joint axes)

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JOINT	X-REACTION	Y-REACTION	Z-MOMENT
1	0.00000	8092.80000	0.00000
2	0.00000	22255.20000	0.00000
3	0.00000	22255.20000	0.00000
4	0.00000	8092.80000	0.00000

MEMBER INTERNAL FORCES (at even intervals along span length)

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SPAN 1	AXIAL	SHEAR	MOMENT
0	0.00000	8092.80000	0.00000
1	0.00000	6069.60000	203938.56000
2	0.00000	4046.40000	349608.96000
3	0.00000	2023.20000	437011.20000
4	0.00000	0.00000	466145.28000
5	0.00000	-2023.20000	437011.20000

6	0.00000	-4046.40000	349608.96000
7	0.00000	-6069.60000	203938.56000
8	0.00000	-8092.80000	0.00000
9	0.00000	-10116.00000	-262206.72000
10	0.00000	-12139.20000	-582681.60000

SPAN 2

0	0.00000	10116.00000	-582681.60000
1	0.00000	8092.80000	-320474.88000
2	0.00000	6069.60000	-116536.32000
3	0.00000	4046.40000	29134.08000
4	0.00000	2023.20000	116536.32000
5	0.00000	0.00000	145670.40000
6	0.00000	-2023.20000	116536.32000
7	0.00000	-4046.40000	29134.08000
8	0.00000	-6069.60000	-116536.32000
9	0.00000	-8092.80000	-320474.88000
10	0.00000	-10116.00000	-582681.60000

SPAN 3

0	0.00000	12139.20000	-582681.60000
1	0.00000	10116.00000	-262206.72000
2	0.00000	8092.80000	0.00000
3	0.00000	6069.60000	203938.56000
4	0.00000	4046.40000	349608.96000
5	0.00000	2023.20000	437011.20000
6	0.00000	0.00000	466145.28000
7	0.00000	-2023.20000	437011.20000
8	0.00000	-4046.40000	349608.96000
9	0.00000	-6069.60000	203938.56000
10	0.00000	-8092.80000	0.00000

SUPPORT JOINT DISPLACEMENTS (in direction of rotated joint axes)

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JOINT	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
1	0.000000000	0.000000000	-0.000527665
2	0.000000000	0.000000000	0.000175888
3	0.000000000	0.000000000	-0.000175888
4	0.000000000	0.000000000	0.000527665

MEMBER DISPLACEMENTS (at even intervals along span length)

=====

SPAN 1	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
0	0.000000000	0.000000000	-0.000527665
1	0.000000000	-0.014816825	-0.000488969
2	0.000000000	-0.027556762	-0.000386954
3	0.000000000	-0.036700136	-0.000242726
4	0.000000000	-0.041335143	-0.000077391
5	0.000000000	-0.041157847	0.000087944
6	0.000000000	-0.036472185	0.000232172
7	0.000000000	-0.028189960	0.000334188
8	0.000000000	-0.017830846	0.000372883
9	0.000000000	-0.007522388	0.000327152
10	0.000000000	0.000000000	0.000175888
SPAN 2			
0	0.000000000	0.000000000	0.000175888
1	0.000000000	0.002507463	0.000014071
2	0.000000000	0.001620986	-0.000063320

3	0.000000000	-0.000531886	-0.000077391
4	0.000000000	-0.002431479	-0.000049249
5	0.000000000	-0.003165988	0.000000000
6	0.000000000	-0.002431479	0.000049249
7	0.000000000	-0.000531886	0.000077391
8	0.000000000	0.001620986	0.000063320
9	0.000000000	0.002507463	-0.000014071
10	0.000000000	0.000000000	-0.000175888

SPAN 3

0	0.000000000	0.000000000	-0.000175888
1	0.000000000	-0.007522388	-0.000327152
2	0.000000000	-0.017830846	-0.000372883
3	0.000000000	-0.028189960	-0.000334188
4	0.000000000	-0.036472185	-0.000232172
5	0.000000000	-0.041157847	-0.000087944
6	0.000000000	-0.041335143	0.000077391
7	0.000000000	-0.036700136	0.000242726
8	0.000000000	-0.027556762	0.000386954
9	0.000000000	-0.014816825	0.000488969
10	0.000000000	0.000000000	0.000527665

MAXIMUM / MINIMUM SPAN SHEARS

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SPAN	MAX SHEAR	LOCATION	MIN SHEAR	LOCATION
1	8092.80000	0.0000	-12139.20000	288.0000
2	10116.00000	0.0000	-10116.00000	288.0000
3	12139.20000	0.0000	-8092.80000	288.0000

MAXIMUM / MINIMUM SPAN MOMENTS

=====

SPAN	MAX MOMENT	LOCATION	MIN MOMENT	LOCATION
1	466145.28000	115.2000	-582681.60000	288.0000
2	145670.40000	144.0000	-582681.60000	0.0000
3	466145.28000	172.8000	-582681.60000	0.0000

MAXIMUM / MINIMUM VERTICAL SPAN DEFLECTIONS

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SPAN	MAX DEFLECTION	LOCATION	MIN DEFLECTION	LOCATION
1	0.000000000	0.0000	-0.041846955	128.6400
2	0.002532784	32.5161	-0.003165988	144.0000
3	0.000000000	0.0000	-0.041846955	159.3600

Roof Framing – East Face: 1 Span (32ft); Supports (Pinned, Pinned)

$F'c = 4,000 \text{ psi}$

$E_c = 3,834,253 \text{ psi}$

$\delta = 150 \text{ lb/ft}^3 \text{ or } 0.0868 \text{ lb/in}^3$

$I = 20,736 \text{ in}^3$

$w = 843 \text{ lb/ft or } 70.25 \text{ lb/in}$

Force = [lb] or [lb*in]

Displacement = [in] or [rad]

Location = [in]

SUPPORT JOINT REACTIONS (in direction of rotated joint axes)

=====

JOINT	X-REACTION	Y-REACTION	Z-MOMENT
1	0.00000	13488.00000	0.00000
2	0.00000	13488.00000	0.00000

MEMBER INTERNAL FORCES (at even intervals along span length)

=====

SPAN 1	AXIAL	SHEAR	MOMENT
0	0.00000	13488.00000	0.00000
1	0.00000	10790.40000	466145.28000
2	0.00000	8092.80000	828702.72000
3	0.00000	5395.20000	1087672.32000
4	0.00000	2697.60000	1243054.08000
5	0.00000	0.00000	1294848.00000

6	0.00000	-2697.60000	1243054.08000
7	0.00000	-5395.20000	1087672.32000
8	0.00000	-8092.80000	828702.72000
9	0.00000	-10790.40000	466145.28000
10	0.00000	-13488.00000	0.00000

SUPPORT JOINT DISPLACEMENTS (in direction of rotated joint axes)

=====

JOINT	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
1	0.000000000	0.000000000	-0.002084600
2	0.000000000	0.000000000	0.002084600

MEMBER DISPLACEMENTS (at even intervals along span length)

=====

SPAN 1	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
0	0.000000000	0.000000000	-0.002084600
1	0.000000000	-0.078527704	-0.001967862
2	0.000000000	-0.148570254	-0.001651003
3	0.000000000	-0.203403564	-0.001184053
4	0.000000000	-0.238224718	-0.000617042
5	0.000000000	-0.250151963	0.000000000
6	0.000000000	-0.238224718	0.000617042
7	0.000000000	-0.203403564	0.001184053
8	0.000000000	-0.148570254	0.001651003
9	0.000000000	-0.078527704	0.001967862
10	0.000000000	0.000000000	0.002084600

MAXIMUM / MINIMUM SPAN SHEARS

=====

SPAN	MAX SHEAR	LOCATION	MIN SHEAR	LOCATION
1	13488.00000	0.0000	-13488.00000	384.0000

MAXIMUM / MINIMUM SPAN MOMENTS

=====

SPAN	MAX MOMENT	LOCATION	MIN MOMENT	LOCATION
1	1294848.00000	192.0000	0.00000	384.0000

MAXIMUM / MINIMUM VERTICAL SPAN DEFLECTIONS

=====

SPAN	MAX DEFLECTION	LOCATION	MIN DEFLECTION	LOCATION
1	0.000000000	0.0000	-0.250151963	192.0000

Roof Framing – North Face: Grid 3.4 – 5.9

$F'c = 4,000 \text{ psi}$

$Ec = 3,834,253 \text{ psi}$

$\delta = 150 \text{ lb/ft}^3 \text{ or } 0.0868 \text{ lb/in}^3$

$I = 27,648 \text{ in}^3$

$w = 2,565 \text{ lb/ft or } 213.8 \text{ lb/in \& } 2,174 \text{ lb/ft or } 181.2 \text{ lb/in}$

Force = [lb] or [lb*in]

Displacement = [in] or [rad]

Location = [in]

SUPPORT JOINT REACTIONS (in direction of rotated joint axes)

=====

JOINT	X-REACTION	Y-REACTION	Z-MOMENT
1	0.00000	0.00000	0.00000
2	0.00000	64415.19943	0.00000
3	0.00000	69991.74021	0.00000
4	0.00000	35805.40236	0.00000
5	0.00000	0.00000	0.00000

MEMBER INTERNAL FORCES (at even intervals along span length)

=====

SPAN 1	AXIAL	SHEAR	MOMENT
0	0.00000	0.00000	0.00000
1	0.00000	-15454.96000	-133002.73600
2	0.00000	-17421.92000	-284236.38400

3	0.00000	-19388.88000	-453566.06400
4	0.00000	-21355.84000	-640991.77600
5	0.00000	-23322.80000	-846513.52000
6	0.00000	-25289.76000	-1070131.29600
7	0.00000	-27256.72000	-1311845.10400
8	0.00000	-29223.68000	-1571654.94400
9	0.00000	-31190.64000	-1849560.81600
10	0.00000	-33157.60000	-2145562.72000

SPAN 2

0	0.00000	31257.59943	-2145562.72000
1	0.00000	25100.15943	-1334010.99242
2	0.00000	18942.71943	-699793.53684
3	0.00000	12785.27943	-242910.35325
4	0.00000	6627.83943	36638.55833
5	0.00000	470.39943	138853.19791
6	0.00000	-5687.04057	63733.56549
7	0.00000	-11844.48057	-188720.33892
8	0.00000	-18001.92057	-618508.51534
9	0.00000	-24159.36057	-1225630.96376
10	0.00000	-30316.80057	-2010087.68418

SPAN 3

0	0.00000	39674.93964	-2010087.68418
1	0.00000	31980.27764	-720255.29998
2	0.00000	24283.47764	292492.29491
3	0.00000	16586.67764	1028155.08979
4	0.00000	8889.87764	1486733.08468
5	0.00000	1193.07764	1668226.27957
6	0.00000	-5721.32236	1582023.47445
7	0.00000	-12244.52236	1258638.26934

8	0.00000	-18749.60236	700418.77023
9	0.00000	-18749.60236	25433.08511
10	0.00000	-18749.60236	-649552.60000

SPAN 4

0	0.00000	17055.80000	-649552.60000
1	0.00000	16077.32000	-560093.17600
2	0.00000	15098.84000	-475917.54400
3	0.00000	14120.36000	-397025.70400
4	0.00000	13141.88000	-323417.65600
5	0.00000	12163.40000	-255093.40000
6	0.00000	11184.92000	-192052.93600
7	0.00000	10206.44000	-134296.26400
8	0.00000	9227.96000	-81823.38400
9	0.00000	8249.48000	-34634.29600
10	0.00000	0.00000	0.00000

SUPPORT JOINT DISPLACEMENTS (in direction of rotated joint axes)

=====

JOINT	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
1	0.000000000	-0.128888765	0.001645827
2	0.000000000	0.000000000	0.000845747
3	0.000000000	0.000000000	-0.000784405
4	0.000000000	0.000000000	0.001309902
5	0.000000000	0.064770736	0.001151526

MEMBER DISPLACEMENTS (at even intervals along span length)

=====

SPAN 1 X-DISPLACEMENT Y-DISPLACEMENT Z-ROTATION

0	0.000000000	-0.128888765	0.001645827
1	0.000000000	-0.113764219	0.001640192
2	0.000000000	-0.098747069	0.001622218
3	0.000000000	-0.083958063	0.001590334
4	0.000000000	-0.069532397	0.001542969
5	0.000000000	-0.055619715	0.001478554
6	0.000000000	-0.042384110	0.001395517
7	0.000000000	-0.030004123	0.001292288
8	0.000000000	-0.018672743	0.001167297
9	0.000000000	-0.008597407	0.001018974
10	0.000000000	0.000000000	0.000845747

SPAN 2

0	0.000000000	0.000000000	0.000845747
1	0.000000000	0.017079938	0.000377107
2	0.000000000	0.023606668	0.000104855
3	0.000000000	0.024542442	-0.000019184
4	0.000000000	0.023462010	-0.000043188
5	0.000000000	0.022552620	-0.000015335
6	0.000000000	0.022614022	0.000016198
7	0.000000000	0.023058463	0.000003235
8	0.000000000	0.021910692	-0.000102402
9	0.000000000	0.015807955	-0.000348889
10	0.000000000	0.000000000	-0.000784405

SPAN 3

0	0.000000000	0.000000000	-0.000784405
1	0.000000000	-0.037756315	-0.001240165
2	0.000000000	-0.084600275	-0.001304957
3	0.000000000	-0.128150708	-0.001072874

4	0.000000000	-0.159413896	-0.000638015
5	0.000000000	-0.172783573	-0.000094473
6	0.000000000	-0.166036672	0.000464363
7	0.000000000	-0.140188527	0.000953342
8	0.000000000	-0.099192361	0.001292628
9	0.000000000	-0.049751644	0.001415875
10	0.000000000	0.000000000	0.001309902

SPAN 4

0	0.000000000	0.000000000	0.001309902
1	0.000000000	0.006977929	0.001275368
2	0.000000000	0.013782905	0.001245795
3	0.000000000	0.020440902	0.001220881
4	0.000000000	0.026976260	0.001200323
5	0.000000000	0.033411693	0.001183821
6	0.000000000	0.039768280	0.001171071
7	0.000000000	0.046065473	0.001161773
8	0.000000000	0.052321094	0.001155624
9	0.000000000	0.058551332	0.001152322
10	0.000000000	0.064770736	0.001151526

MAXIMUM / MINIMUM SPAN SHEARS

=====

SPAN	MAX SHEAR	LOCATION	MIN SHEAR	LOCATION
1	0.00000	0.0000	-33157.60000	92.0000
2	31257.59943	0.0000	-30316.80057	288.0000
3	39674.93964	0.0000	-18749.60236	302.4000
4	17055.80000	0.0000	0.00000	54.0000

MAXIMUM / MINIMUM SPAN MOMENTS

=====

SPAN	MAX MOMENT	LOCATION	MIN MOMENT	LOCATION
1	0.00000	0.0000	-2145562.72000	92.0000
2	139364.69907	145.9636	-2145562.72000	0.0000
3	1671544.86108	185.8909	-2010087.68418	0.0000
4	0.00000	54.0000	-649552.60000	0.0000

MAXIMUM / MINIMUM VERTICAL SPAN DEFLECTIONS

=====

SPAN	MAX DEFLECTION	LOCATION	MIN DEFLECTION	LOCATION
1	0.000000000	92.0000	-0.128888765	0.0000
2	0.024613445	79.2000	0.000000000	0.0000
3	0.000000000	0.0000	-0.173066770	185.8909
4	0.064770736	54.0000	0.000000000	0.0000 0.0000 3 192.0000

Roof Framing – South Face: Grid 4-8

$F'c = 4,000 \text{ psi}$

$E_c = 3,834,253 \text{ psi}$

$\delta = 150 \text{ lb/ft}^3 \text{ or } 0.0868 \text{ lb/in}^3$

$I = 27,648 \text{ in}^3$

$w = 2,565 \text{ lb/ft or } 213.8 \text{ lb/in}$

Force = [lb] or [lb*in]

Displacement = [in] or [rad]

Location = [in]

SUPPORT JOINT REACTIONS (in direction of rotated joint axes)

These results are stored in the files named:

C:\Program Files (x86)\DTBeam\DTBeam.dtb.txt

C:\Program Files (x86)\DTBeam\DTBeam.dtb.bmp

Force = [lb] or [lb*in]

Displacement = [in] or [rad]

Location = [in]

SUPPORT JOINT REACTIONS (in direction of rotated joint axes)

=====

JOINT	X-REACTION	Y-REACTION	Z-MOMENT
1	0.00000	0.00000	0.00000
2	0.00000	38500.48382	0.00000

3	0.00000	64170.30012	0.00000
4	0.00000	67323.53636	0.00000
5	0.00000	64194.68112	0.00000
6	0.00000	44632.19858	0.00000
7	0.00000	0.00000	0.00000

MEMBER INTERNAL FORCES (at even intervals along span length)

=====

SPAN 1	AXIAL	SHEAR	MOMENT
0	0.00000	0.00000	0.00000
1	0.00000	-8734.40000	-25160.17000
2	0.00000	-9375.80000	-52325.47000
3	0.00000	-10017.20000	-81414.97000
4	0.00000	-10658.60000	-112428.67000
5	0.00000	-11300.00000	-145366.57000
6	0.00000	-11941.40000	-180228.67000
7	0.00000	-12582.80000	-217014.97000
8	0.00000	-13224.20000	-255725.47000
9	0.00000	-13865.60000	-296360.17000
10	0.00000	-14507.00000	-338919.07000
SPAN 2			
0	0.00000	23993.48382	-338919.07000
1	0.00000	18263.64382	227326.44047
2	0.00000	12533.80382	640012.23894
3	0.00000	6803.96382	899138.32541
4	0.00000	1074.12382	1004704.69989
5	0.00000	-4655.71618	956711.36236
6	0.00000	-10385.55618	755158.31283

7	0.00000	-16115.39618	400045.55130
8	0.00000	-21845.23618	-108626.92223
9	0.00000	-27575.07618	-770859.10776
10	0.00000	-33304.91618	-1586651.00528

SPAN 3

0	0.00000	30865.38395	-1586651.00528
1	0.00000	24707.94395	-786395.08359
2	0.00000	18550.50395	-163473.43390
3	0.00000	12393.06395	282113.94380
4	0.00000	6235.62395	550367.04949
5	0.00000	78.18395	641285.88319
6	0.00000	-6079.25605	554870.44488
7	0.00000	-12236.69605	291120.73457
8	0.00000	-18394.13605	-149963.24773
9	0.00000	-24551.57605	-768381.50204
10	0.00000	-30709.01605	-1564134.02834

SPAN 4

0	0.00000	36614.52030	-1564134.02834
1	0.00000	30457.08030	-598302.97961
2	0.00000	17028.64030	156747.19712
3	0.00000	10871.20030	558504.90185
4	0.00000	4713.76030	782928.33458
5	0.00000	-1443.67970	830017.49531
6	0.00000	-7601.11970	699772.38404
7	0.00000	-13758.55970	392193.00077
8	0.00000	-19915.99970	-92720.65450
9	0.00000	-26073.43970	-754968.58177
10	0.00000	-32230.87970	-1594550.78104

SPAN 5

0	0.00000	31963.80142	-1594550.78104
1	0.00000	26233.96142	-814700.75894
2	0.00000	20504.12142	-188410.44884
3	0.00000	14774.28142	284320.14927
4	0.00000	9044.44142	603491.03537
5	0.00000	3314.60142	769102.20948
6	0.00000	-2415.23858	781153.67158
7	0.00000	-8145.07858	639645.42169
8	0.00000	-13874.91858	344577.45979
9	0.00000	-19604.75858	-104050.21410
10	0.00000	-25334.59858	-706237.60000

SPAN 6

0	0.00000	19297.60000	-706237.60000
1	0.00000	18185.84000	-608780.65600
2	0.00000	17074.08000	-517104.86400
3	0.00000	15962.32000	-431210.22400
4	0.00000	14850.56000	-351096.73600
5	0.00000	13738.80000	-276764.40000
6	0.00000	12627.04000	-208213.21600
7	0.00000	11515.28000	-145443.18400
8	0.00000	10403.52000	-88454.30400
9	0.00000	9291.76000	-37246.57600
10	0.00000	0.00000	0.00000

SUPPORT JOINT DISPLACEMENTS (in direction of rotated joint axes)

=====

JOINT X-DISPLACEMENT Y-DISPLACEMENT Z-ROTATION

1 0.000000000 0.019011395 -0.000620004

2	0.000000000	0.000000000	-0.000663411
3	0.000000000	0.000000000	0.000137684
4	0.000000000	0.000000000	-0.000127489
5	0.000000000	0.000000000	0.000023741
6	0.000000000	0.000000000	0.000350546
7	0.000000000	0.012872869	0.000202933

MEMBER DISPLACEMENTS (at even intervals along span length)

=====

SPAN 1	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
0	0.000000000	0.019011395	-0.000620004
1	0.000000000	0.017151035	-0.000620355
2	0.000000000	0.015288525	-0.000621447
3	0.000000000	0.013421559	-0.000623334
4	0.000000000	0.011547668	-0.000626073
5	0.000000000	0.009664218	-0.000629716
6	0.000000000	0.007768414	-0.000634318
7	0.000000000	0.005857294	-0.000639935
8	0.000000000	0.003927737	-0.000646619
9	0.000000000	0.001976455	-0.000654427
10	0.000000000	0.000000000	-0.000663411

SPAN 2

0	0.000000000	0.000000000	-0.000663411
1	0.000000000	-0.018244788	-0.000674282
2	0.000000000	-0.035036085	-0.000561412
3	0.000000000	-0.047577840	-0.000363622
4	0.000000000	-0.054114413	-0.000119734
5	0.000000000	-0.053930564	0.000131432

6	0.000000000	-0.047351460	0.000351054
7	0.000000000	-0.035742672	0.000500311
8	0.000000000	-0.021510178	0.000540383
9	0.000000000	-0.008100359	0.000432447
10	0.000000000	0.000000000	0.000137684

SPAN 3

0	0.000000000	0.000000000	0.000137684
1	0.000000000	-0.001140471	-0.000180649
2	0.000000000	-0.008549488	-0.000305661
3	0.000000000	-0.017353180	-0.000285531
4	0.000000000	-0.024065178	-0.000168434
5	0.000000000	-0.026586614	-0.000002549
6	0.000000000	-0.024206120	0.000163948
7	0.000000000	-0.017599828	0.000282880
8	0.000000000	-0.008831372	0.000306069
9	0.000000000	-0.001351884	0.000185339
10	0.000000000	0.000000000	-0.000127489

SPAN 4

0	0.000000000	0.000000000	-0.000127489
1	0.000000000	-0.008473437	-0.000417213
2	0.000000000	-0.021744859	-0.000469360
3	0.000000000	-0.034067495	-0.000368187
4	0.000000000	-0.042135896	-0.000181956
5	0.000000000	-0.044194127	0.000041156
6	0.000000000	-0.039873751	0.000252973
7	0.000000000	-0.030193835	0.000405317
8	0.000000000	-0.017560944	0.000450011
9	0.000000000	-0.005769143	0.000338878
10	0.000000000	0.000000000	0.000023741

SPAN 5

0	0.000000000	0.000000000	0.000023741
1	0.000000000	-0.003841507	-0.000277562
2	0.000000000	-0.013289513	-0.000401124
3	0.000000000	-0.024100746	-0.000385766
4	0.000000000	-0.033072341	-0.000270308
5	0.000000000	-0.038041836	-0.000093572
6	0.000000000	-0.037887174	0.000105622
7	0.000000000	-0.032526703	0.000288451
8	0.000000000	-0.022919178	0.000416096
9	0.000000000	-0.011063756	0.000449734
10	0.000000000	0.000000000	0.000350546

SPAN 6

0	0.000000000	0.000000000	0.000350546
1	0.000000000	0.001736973	0.000318317
2	0.000000000	0.003318541	0.000290727
3	0.000000000	0.004768087	0.000267493
4	0.000000000	0.006107521	0.000248329
5	0.000000000	0.007357277	0.000232954
6	0.000000000	0.008536315	0.000221083
7	0.000000000	0.009662121	0.000212433
8	0.000000000	0.010750706	0.000206720
9	0.000000000	0.011816606	0.000203660
10	0.000000000	0.012872869	0.000202933

MAXIMUM / MINIMUM SPAN SHEARS

=====

SPAN	MAX SHEAR	LOCATION	MIN SHEAR	LOCATION
------	-----------	----------	-----------	----------

1	0.00000	0.0000	-14507.00000	30.0000
2	23993.48382	0.0000	-33304.91618	268.0000
3	30865.38395	0.0000	-30709.01605	288.0000
4	36614.52030	0.0000	-32230.87970	288.0000
5	31963.80142	0.0000	-25334.59858	268.0000
6	19297.60000	0.0000	0.00000	52.0000

MAXIMUM / MINIMUM SPAN MOMENTS

=====

SPAN	MAX MOMENT	LOCATION	MIN MOMENT	LOCATION
1	0.00000	0.0000	-338919.07000	30.0000
2	1007402.48259	112.1630	-1586651.00528	268.0000
3	641285.88319	144.0000	-1586651.00528	0.0000
4	834887.45882	137.0483	-1594550.78104	288.0000
5	794780.51714	149.8815	-1594550.78104	0.0000
6	0.00000	52.0000	-706237.60000	0.0000

MAXIMUM / MINIMUM VERTICAL SPAN DEFLECTIONS

=====

SPAN	MAX DEFLECTION	LOCATION	MIN DEFLECTION	LOCATION
1	0.019011395	0.0000	0.000000000	30.0000
2	0.000000000	0.0000	-0.054869096	120.1037
3	0.000675989	9.9310	-0.026586614	144.0000
4	0.000000000	0.0000	-0.044301735	139.0345
5	0.000017884	1.9852	-0.038636390	146.9037
6	0.012872869	52.0000	0.000000000	0.0000

Roof Framing – West Face: 2 Spans (16ft, 24ft); Supports (Fixed, Pinned, Pinned)

$F'c = 4,000$ psi

$Ec = 3,834,253$ psi

$\delta = 150$ lb/ft³ or 0.0868 lb/in³

$I = 20,736$ in³

$w = 843$ lb/ft or 70.25 lb/in

Force = [lb] or [lb*in]

Displacement = [in] or [rad]

Location = [in]

SUPPORT JOINT REACTIONS (in direction of rotated joint axes)

=====

JOINT	X-REACTION	Y-REACTION	Z-MOMENT
1	0.00000	4074.50000	44960.00000
2	0.00000	21465.27778	0.00000
3	0.00000	8180.22222	0.00000

MEMBER INTERNAL FORCES (at even intervals along span length)

=====

SPAN 1	AXIAL	SHEAR	MOMENT
0	0.00000	4074.50000	-44960.00000
1	0.00000	2725.70000	20321.92000
2	0.00000	1376.90000	59706.88000
3	0.00000	28.10000	73194.88000
4	0.00000	-1320.70000	60785.92000
5	0.00000	-2669.50000	22480.00000

6	0.00000	-4018.30000	-41722.88000
7	0.00000	-5367.10000	-131822.72000
8	0.00000	-6715.90000	-247819.52000
9	0.00000	-8064.70000	-389713.28000
10	0.00000	-9413.50000	-557504.00000

SPAN 2

0	0.00000	12051.77778	-557504.00000
1	0.00000	10028.57778	-239546.88000
2	0.00000	8005.37778	20142.08000
3	0.00000	5982.17778	221562.88000
4	0.00000	3958.97778	364715.52000
5	0.00000	1935.77778	449600.00000
6	0.00000	-87.42222	476216.32000
7	0.00000	-2110.62222	444564.48000
8	0.00000	-4133.82222	354644.48000
9	0.00000	-6157.02222	206456.32000
10	0.00000	-8180.22222	0.00000

SUPPORT JOINT DISPLACEMENTS (in direction of rotated joint axes)

=====

JOINT	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
1	0.000000000	0.000000000	0.000000000
2	0.000000000	0.000000000	-0.000206289
3	0.000000000	0.000000000	0.000542865

MEMBER DISPLACEMENTS (at even intervals along span length)

=====

SPAN 1	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
0	0.000000000	0.000000000	0.000000000
1	0.000000000	-0.000048780	-0.000002454
2	0.000000000	-0.000013341	0.000007730
3	0.000000000	0.000288926	0.000024299
4	0.000000000	0.000920560	0.000040997
5	0.000000000	0.001824026	0.000051572
6	0.000000000	0.002821716	0.000049770
7	0.000000000	0.003615949	0.000029336
8	0.000000000	0.003788971	-0.000015982
9	0.000000000	0.002802955	-0.000092439
10	0.000000000	0.000000000	-0.000206289

SPAN 2

0	0.000000000	0.000000000	-0.000206289
1	0.000000000	-0.008270968	-0.000348888
2	0.000000000	-0.019091613	-0.000386867
3	0.000000000	-0.029752785	-0.000341332
4	0.000000000	-0.038153207	-0.000233388
5	0.000000000	-0.042799471	-0.000084144
6	0.000000000	-0.042806037	0.000085295
7	0.000000000	-0.037895238	0.000253822
8	0.000000000	-0.028397273	0.000400330
9	0.000000000	-0.015250214	0.000503714
10	0.000000000	0.000000000	0.000542865

MAXIMUM / MINIMUM SPAN SHEARS

=====

SPAN	MAX SHEAR	LOCATION	MIN SHEAR	LOCATION
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1	4074.50000	0.0000	-9413.50000	192.0000
2	12051.77778	0.0000	-8180.22222	288.0000

MAXIMUM / MINIMUM SPAN MOMENTS

=====

SPAN	MAX MOMENT	LOCATION	MIN MOMENT	LOCATION
1	73195.74323	58.3680	-557504.00000	192.0000
2	476267.28999	171.2432	-557504.00000	0.0000

MAXIMUM / MINIMUM VERTICAL SPAN DEFLECTIONS

=====

SPAN	MAX DEFLECTION	LOCATION	MIN DEFLECTION	LOCATION
1	0.003832005	148.2240	-0.000057831	26.1120
2	0.000000000	0.0000	-0.043414115	158.7892

Roof Framing – West Face: 1 Span (23ft); Supports (Pinned, Fixed)

$F'c = 4,000 \text{ psi}$

$Ec = 3,834,253 \text{ psi}$

$\delta = 150 \text{ lb/ft}^3 \text{ or } 0.0868 \text{ lb/in}^3$

$I = 20,736 \text{ in}^3$

$w = 843 \text{ lb/ft or } 70.25 \text{ lb/in}$

Force = [lb] or [lb*in]

Displacement = [in] or [rad]

Location = [in]

SUPPORT JOINT REACTIONS (in direction of rotated joint axes)

=====

JOINT	X-REACTION	Y-REACTION	Z-MOMENT
1	0.00000	7270.87500	0.00000
2	0.00000	12118.12500	-668920.50000

MEMBER INTERNAL FORCES (at even intervals along span length)

=====

SPAN 1	AXIAL	SHEAR	MOMENT
0	0.00000	7270.87500	0.00000
1	0.00000	5331.97500	173919.33000
2	0.00000	3393.07500	294325.02000
3	0.00000	1454.17500	361217.07000
4	0.00000	-484.72500	374595.48000
5	0.00000	-2423.62500	334460.25000

6	0.00000	-4362.52500	240811.38000
7	0.00000	-6301.42500	93648.87000
8	0.00000	-8240.32500	-107027.28000
9	0.00000	-10179.22500	-361217.07000
10	0.00000	-12118.12500	-668920.50000

SUPPORT JOINT DISPLACEMENTS (in direction of rotated joint axes)

=====

JOINT	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
1	0.000000000	0.000000000	-0.000387014
2	0.000000000	0.000000000	0.000000000

MEMBER DISPLACEMENTS (at even intervals along span length)

=====

SPAN 1	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
0	0.000000000	0.000000000	-0.000387014
1	0.000000000	-0.010382500	-0.000355279
2	0.000000000	-0.019141399	-0.000272458
3	0.000000000	-0.025123086	-0.000157128
4	0.000000000	-0.027686666	-0.000027865
5	0.000000000	-0.026703961	0.000096753
6	0.000000000	-0.022559506	0.000198151
7	0.000000000	-0.016150555	0.000257751
8	0.000000000	-0.008887078	0.000256977
9	0.000000000	-0.002691759	0.000177252
10	0.000000000	0.000000000	0.000000000

MAXIMUM / MINIMUM SPAN SHEARS

=====

SPAN	MAX SHEAR	LOCATION	MIN SHEAR	LOCATION
1	7270.87500	0.0000	-12118.12500	276.0000

MAXIMUM / MINIMUM SPAN MOMENTS

=====

SPAN	MAX MOMENT	LOCATION	MIN MOMENT	LOCATION
1	376260.34880	103.9600	-668920.50000	276.0000

MAXIMUM / MINIMUM VERTICAL SPAN DEFLECTIONS

=====

SPAN	MAX DEFLECTION	LOCATION	MIN DEFLECTION	LOCATION
1	0.000000000	0.0000	-0.027768906	115.9200

Roof Framing – West Face: 1 Span (24ft); Supports (Pinned, Fixed)

$F'c = 4,000 \text{ psi}$

$Ec = 3,834,253 \text{ psi}$

$\delta = 150 \text{ lb/ft}^3 \text{ or } 0.0868 \text{ lb/in}^3$

$I = 20,736 \text{ in}^3$

$w = 843 \text{ lb/ft or } 70.25 \text{ lb/in}$

Force = [lb] or [lb*in]

Displacement = [in] or [rad]

Location = [in]

SUPPORT JOINT REACTIONS (in direction of rotated joint axes)

=====

JOINT	X-REACTION	Y-REACTION	Z-MOMENT
1	0.00000	7587.00000	0.00000
2	0.00000	12645.00000	-728352.00000

MEMBER INTERNAL FORCES (at even intervals along span length)

=====

SPAN 1	AXIAL	SHEAR	MOMENT
0	0.00000	7587.00000	0.00000
1	0.00000	5563.80000	189371.52000
2	0.00000	3540.60000	320474.88000
3	0.00000	1517.40000	393310.08000
4	0.00000	-505.80000	407877.12000
5	0.00000	-2529.00000	364176.00000
6	0.00000	-4552.20000	262206.72000

7	0.00000	-6575.40000	101969.28000
8	0.00000	-8598.60000	-116536.32000
9	0.00000	-10621.80000	-393310.08000
10	0.00000	-12645.00000	-728352.00000

SUPPORT JOINT DISPLACEMENTS (in direction of rotated joint axes)

=====

JOINT	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
1	0.000000000	0.000000000	-0.000439721
2	0.000000000	0.000000000	0.000000000

MEMBER DISPLACEMENTS (at even intervals along span length)

=====

SPAN 1	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
0	0.000000000	0.000000000	-0.000439721
1	0.000000000	-0.012309362	-0.000403664
2	0.000000000	-0.022693804	-0.000309563
3	0.000000000	-0.029785618	-0.000178527
4	0.000000000	-0.032824966	-0.000031660
5	0.000000000	-0.031659883	0.000109930
6	0.000000000	-0.026746269	0.000225137
7	0.000000000	-0.019147897	0.000292854
8	0.000000000	-0.010536409	0.000291974
9	0.000000000	-0.003191316	0.000201392
10	0.000000000	0.000000000	0.000000000

MAXIMUM / MINIMUM SPAN SHEARS

=====

SPAN	MAX SHEAR	LOCATION	MIN SHEAR	LOCATION
1	7587.00000	0.0000	-12645.00000	288.0000

MAXIMUM / MINIMUM SPAN MOMENTS

=====

SPAN	MAX MOMENT	LOCATION	MIN MOMENT	LOCATION
1	409689.90720	108.4800	-728352.00000	288.0000

MAXIMUM / MINIMUM VERTICAL SPAN DEFLECTIONS

=====

SPAN	MAX DEFLECTION	LOCATION	MIN DEFLECTION	LOCATION
1	0.000000000	0.0000	-0.032922469	120.9600.043414115 158.7892

Roof Framing – West Face: 1 Span (31ft); Supports (Fixed, Fixed)

$F'c = 4,000 \text{ psi}$

$Ec = 3,834,253 \text{ psi}$

$\delta = 150 \text{ lb/ft}^3 \text{ or } 0.0868 \text{ lb/in}^3$

$I = 20,736 \text{ in}^3$

$w = 843 \text{ lb/ft or } 70.25 \text{ lb/in}$

Force = [lb] or [lb*in]

Displacement = [in] or [rad]

Location = [in]

SUPPORT JOINT REACTIONS (in direction of rotated joint axes)

=====

JOINT	X-REACTION	Y-REACTION	Z-MOMENT
1	0.00000	13066.50000	810123.00000
2	0.00000	13066.50000	-810123.00000

MEMBER INTERNAL FORCES (at even intervals along span length)

=====

SPAN 1	AXIAL	SHEAR	MOMENT
0	0.00000	13066.50000	-810123.00000
1	0.00000	10453.20000	-372656.58000
2	0.00000	7839.90000	-32404.92000
3	0.00000	5226.60000	210631.98000
4	0.00000	2613.30000	356454.12000
5	0.00000	0.00000	405061.50000

6	0.00000	-2613.30000	356454.12000
7	0.00000	-5226.60000	210631.98000
8	0.00000	-7839.90000	-32404.92000
9	0.00000	-10453.20000	-372656.58000
10	0.00000	-13066.50000	-810123.00000

SUPPORT JOINT DISPLACEMENTS (in direction of rotated joint axes)

=====

JOINT	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
1	0.000000000	0.000000000	0.000000000
2	0.000000000	0.000000000	0.000000000

MEMBER DISPLACEMENTS (at even intervals along span length)

=====

SPAN 1	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
0	0.000000000	0.000000000	0.000000000
1	0.000000000	-0.005710658	-0.000272911
2	0.000000000	-0.018048498	-0.000363881
3	0.000000000	-0.031091358	-0.000318396
4	0.000000000	-0.040609120	-0.000181941
5	0.000000000	-0.044063715	0.000000000
6	0.000000000	-0.040609120	0.000181941
7	0.000000000	-0.031091358	0.000318396
8	0.000000000	-0.018048498	0.000363881
9	0.000000000	-0.005710658	0.000272911
10	0.000000000	0.000000000	0.000000000

MAXIMUM / MINIMUM SPAN SHEARS

=====

SPAN	MAX SHEAR	LOCATION	MIN SHEAR	LOCATION
1	13066.50000	0.0000	-13066.50000	372.0000

MAXIMUM / MINIMUM SPAN MOMENTS

=====

SPAN	MAX MOMENT	LOCATION	MIN MOMENT	LOCATION
1	405061.50000	186.0000	-810123.00000	372.0000

MAXIMUM / MINIMUM VERTICAL SPAN DEFLECTIONS

=====

SPAN	MAX DEFLECTION	LOCATION	MIN DEFLECTION	LOCATION
1	0.000000000	0.0000	-0.044063715	186.0000

Roof Framing

Perimeter Beam – West Face between Col. Line D-E

Force = [lb] or [lb*in]

Displacement = [in] or [rad]

Location = [in]

SUPPORT JOINT REACTIONS (in direction of rotated joint axes)

=====

JOINT	X-REACTION	Y-REACTION	Z-MOMENT
1	0.00000	10944.00000	0.00000
2	0.00000	10944.00000	0.00000

MEMBER INTERNAL FORCES (at even intervals along span length)

=====

SPAN 1	AXIAL	SHEAR	MOMENT
0	0.00000	10944.00000	0.00000
1	0.00000	8755.20000	283668.48000
2	0.00000	6566.40000	504299.52000
3	0.00000	4377.60000	661893.12000
4	0.00000	2188.80000	756449.28000
5	0.00000	0.00000	787968.00000
6	0.00000	-2188.80000	756449.28000
7	0.00000	-4377.60000	661893.12000
8	0.00000	-6566.40000	504299.52000
9	0.00000	-8755.20000	283668.48000

10 0.00000 -10944.00000 0.00000

SUPPORT JOINT DISPLACEMENTS (in direction of rotated joint axes)

=====

JOINT	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
1	0.000000000	0.000000000	-0.000951424
2	0.000000000	0.000000000	0.000951424

MEMBER DISPLACEMENTS (at even intervals along span length)

=====

SPAN 1	X-DISPLACEMENT	Y-DISPLACEMENT	Z-ROTATION
0	0.000000000	0.000000000	-0.000951424
1	0.000000000	-0.026880390	-0.000898144
2	0.000000000	-0.050856272	-0.000753528
3	0.000000000	-0.069625963	-0.000540409
4	0.000000000	-0.081545402	-0.000281621
5	0.000000000	-0.085628152	0.000000000
6	0.000000000	-0.081545402	0.000281621
7	0.000000000	-0.069625963	0.000540409
8	0.000000000	-0.050856272	0.000753528
9	0.000000000	-0.026880390	0.000898144
10	0.000000000	0.000000000	0.000951424

MAXIMUM / MINIMUM SPAN SHEARS

=====

SPAN	MAX SHEAR	LOCATION	MIN SHEAR	LOCATION
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1 10944.00000 0.0000 -10944.00000 288.0000

MAXIMUM / MINIMUM SPAN MOMENTS

=====

SPAN	MAX MOMENT	LOCATION	MIN MOMENT	LOCATION
1	787968.00000	144.0000	0.00000	288.0000

MAXIMUM / MINIMUM VERTICAL SPAN DEFLECTIONS

=====

SPAN	MAX DEFLECTION	LOCATION	MIN DEFLECTION	LOCATION
1	0.000000000	0.0000	-0.085628152	144.0000